

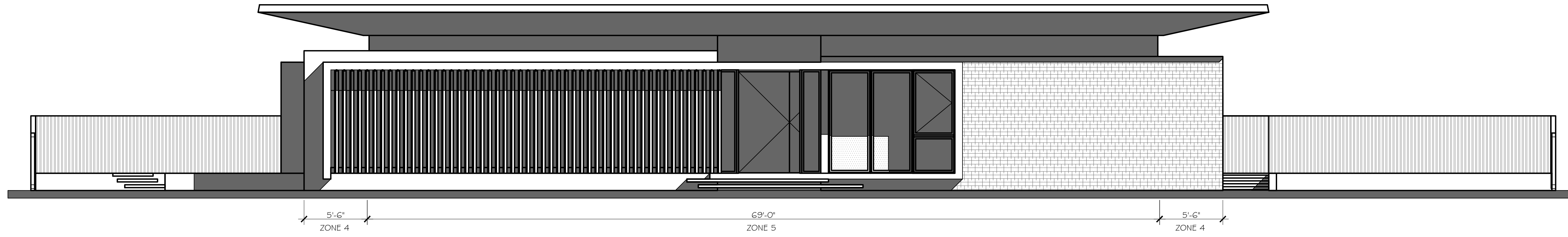
A TYPICAL THICKENED SLAB EDGE (TSE) DETAIL
S101 SCALE: 3/4" = 1'-0"
 NOTE: OTHERWISE NOTED ON SECTION.

- LEGEND:**
- NEW 8" BEARING CMU WALL REINF. W/ #5 REBAR @32" IN C.L OF FILLED CELL.
 - NEW 8" BEARING CMU STEM WALL REINF. W/ #5 REBAR @32" IN C.L OF FILLED CELL.
 - CONC. COL. OR TIE COL. SEE COL. SCHEDULE FOR REINF. & DIM. # TYP. COL. DETAIL ON S301.
 - STEEL COL. SEE COL. SCHEDULE FOR DIM. ON & TYP. DETAIL ON S301.
 - THICKENED SLAB EDGE (TSE). SEE TYP. DETAIL ON THIS SHEET.
 - NEW CONC. WALL FOOTING. SEE FOOTING SCHEDULE ON S301.
 - CONC. ISOLATED FOOTING. SEE FOOTING SCHEDULE ON S301.
 - PROVIDE ADDITIONAL 4x4" TOP MESH AT ALL REENTRANT CORNER (TYP).

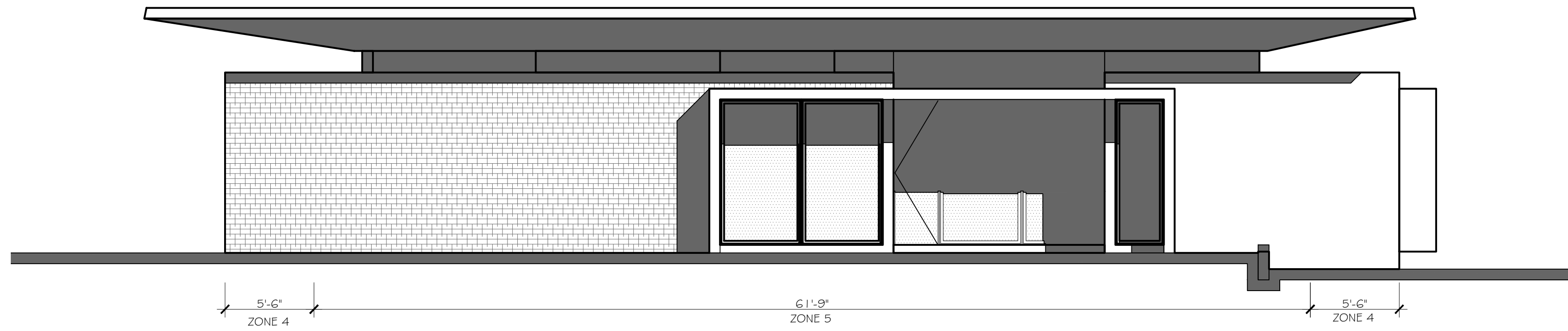
- NOTES:**
1. - TOP OF FOOTING : MATCH TO EXISTING FOOTING ELEVATION OR 2'-6" MIN. UNLESS OTHERWISE NOTED
 2. - ALL DIMS SHOWN MUST BE DOUBLE CHECK W/ ARCH. PLANS. ARCH PLANS SUPERSEDE STRUCTURAL PLANS DIMENSION WISE.

PROPOSED FOUNDATION / GROUND FLOOR FRAMING PLAN
 SCALE: 3/16" = 1'-0"

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FRONT FACADE
SCALE: 3/16" = 1'-0"



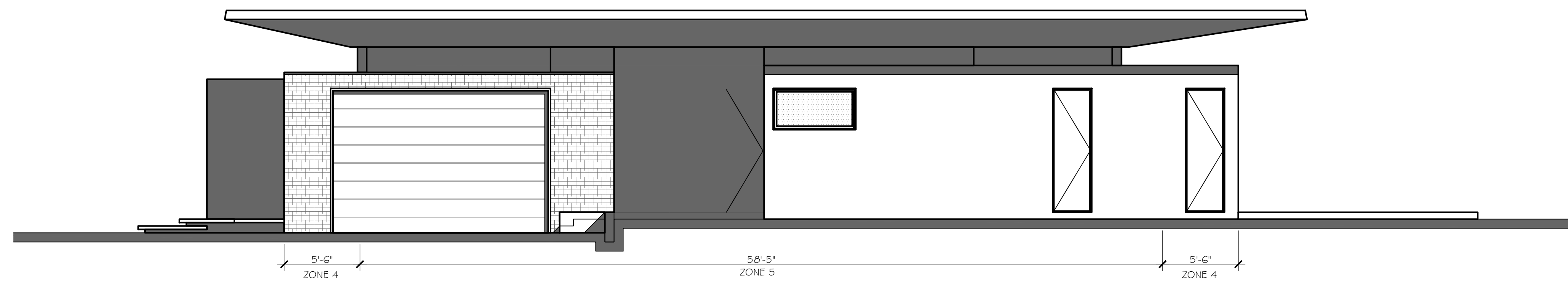
SIDE (L) FACADE
SCALE: 3/16" = 1'-0"

COMPONENT AREA	ZONE 4		ZONE 5	
	PRESSURE	SUCTION	PRESSURE	SUCTION
ALL OPENINGS	44.49	-48.20	44.49	-59.32

NOTE:
1 - WIND PRESSURES FOR A PANEL AREA IN BETWEEN TWO LIMITING AREA VALUES SHALL BE ROUNDED OFF TO THE GREATEST VALUE OF THE WIND PRESSURES SHOWN

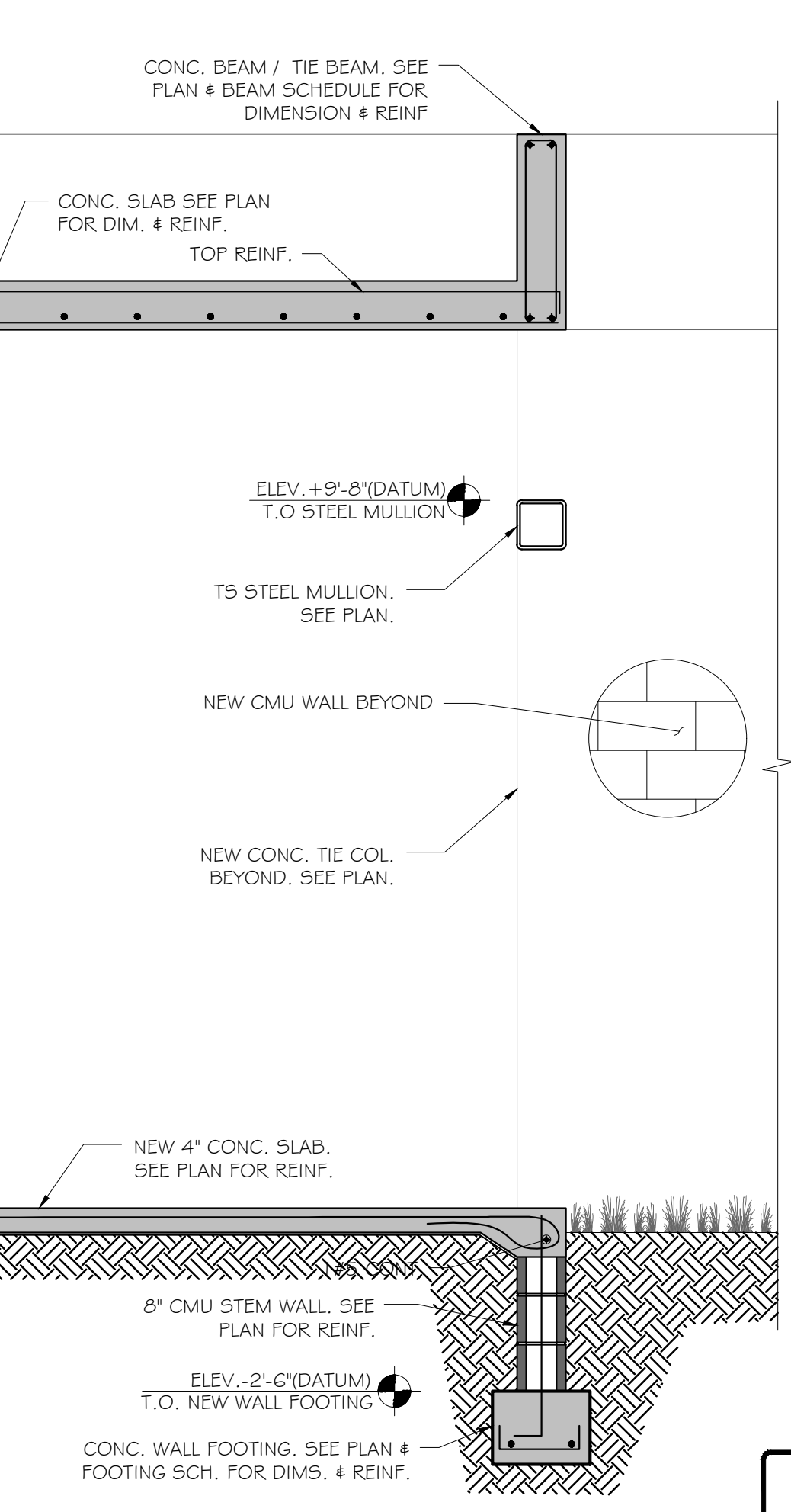
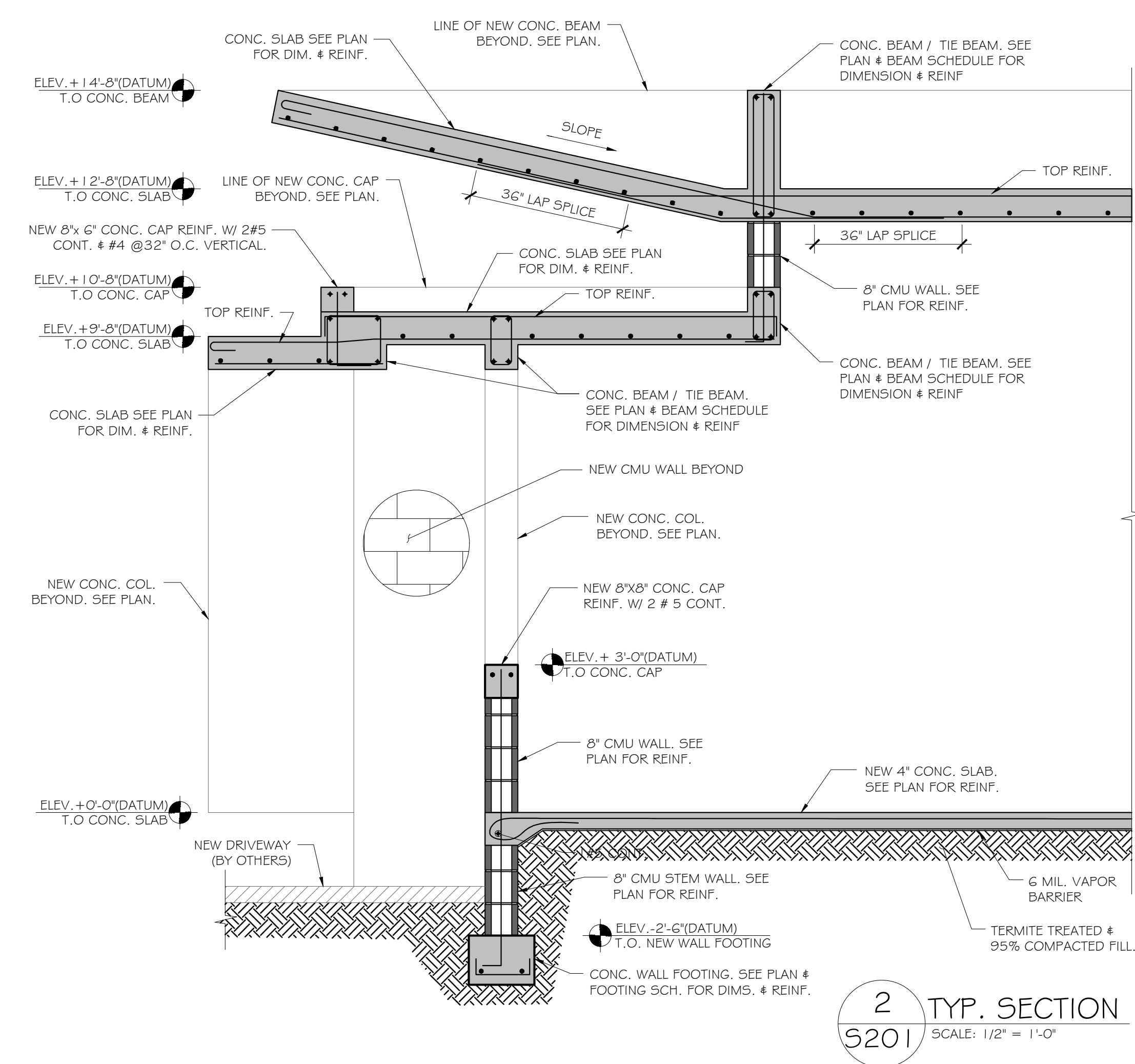
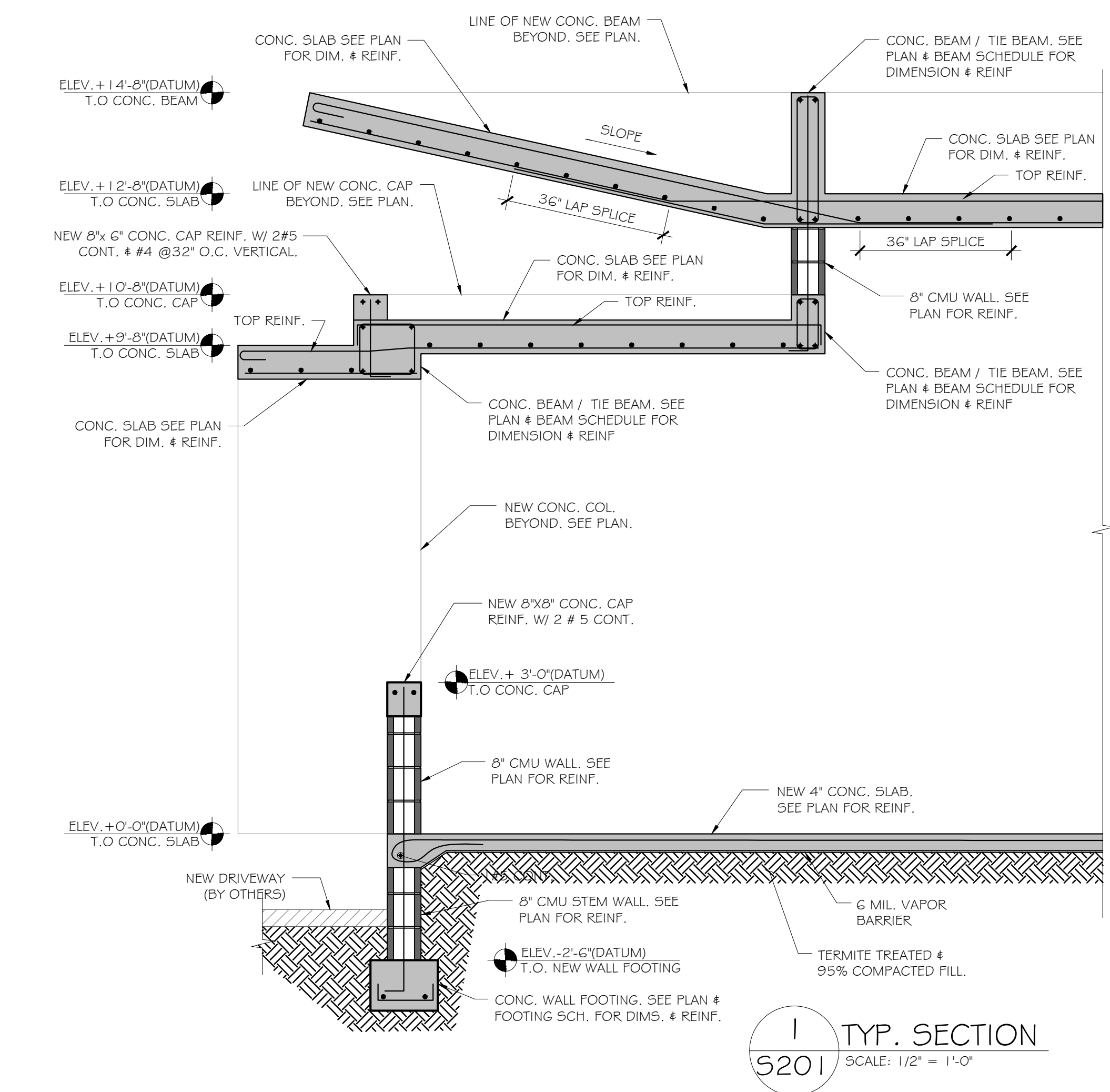
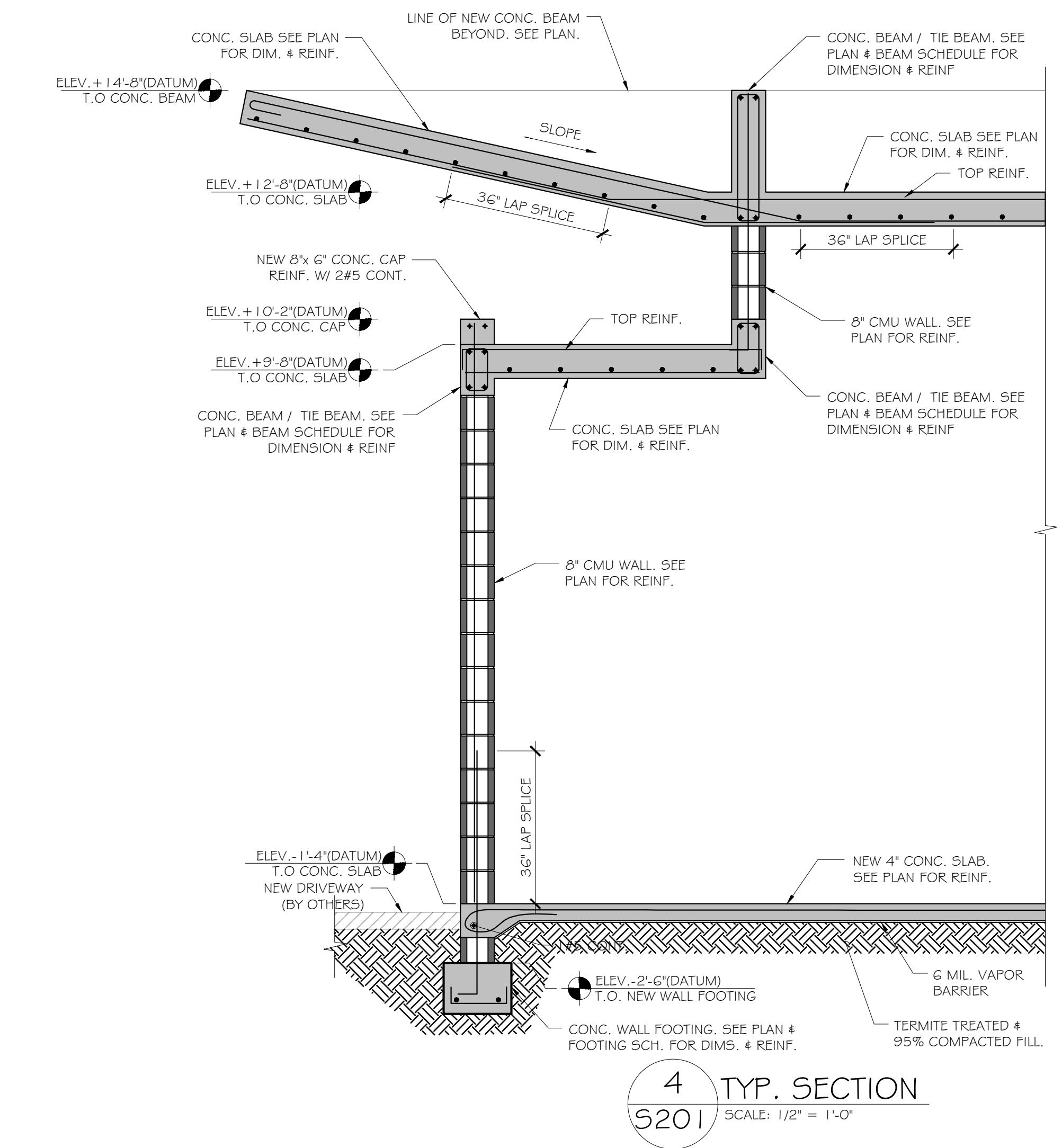
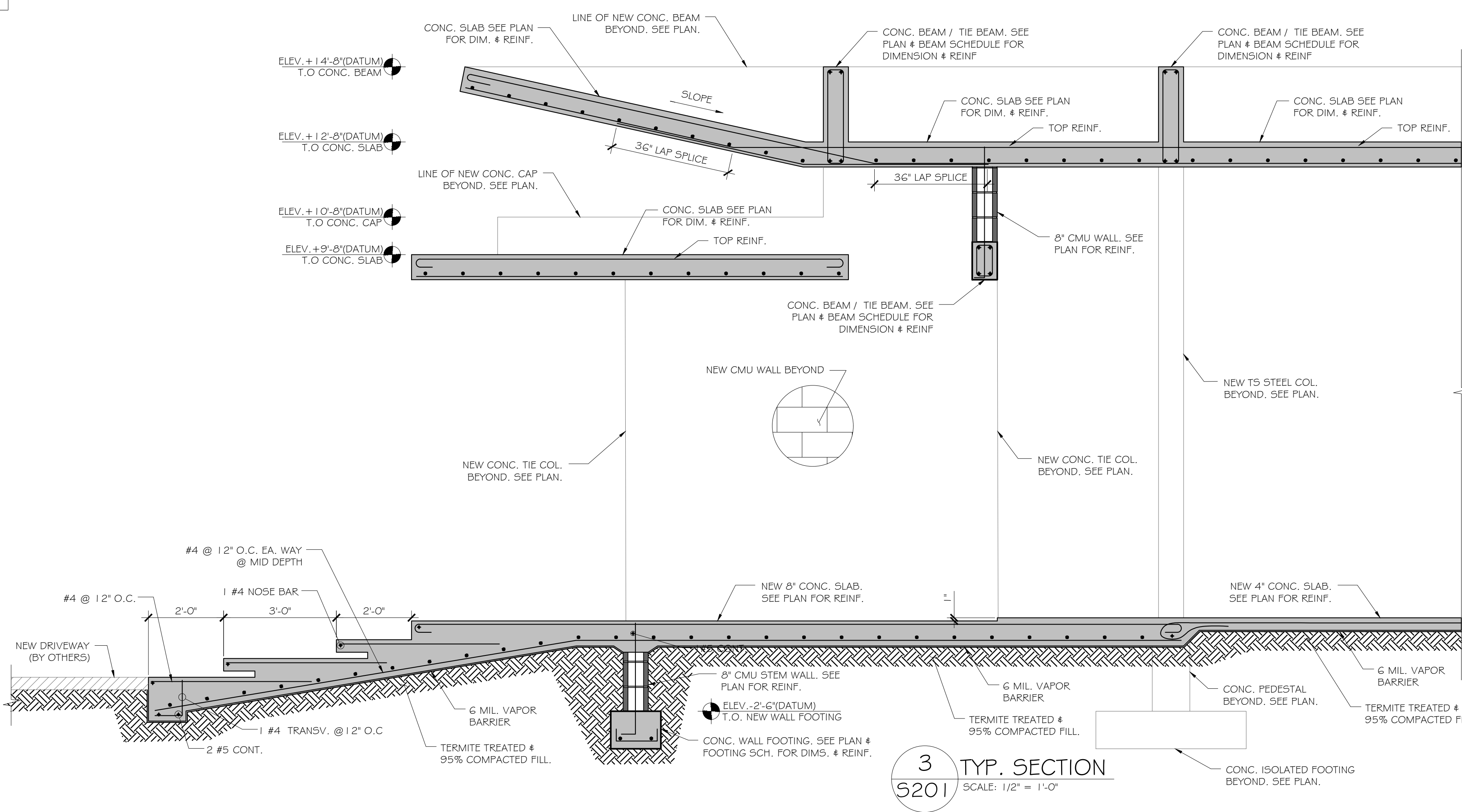


REAR FACADE

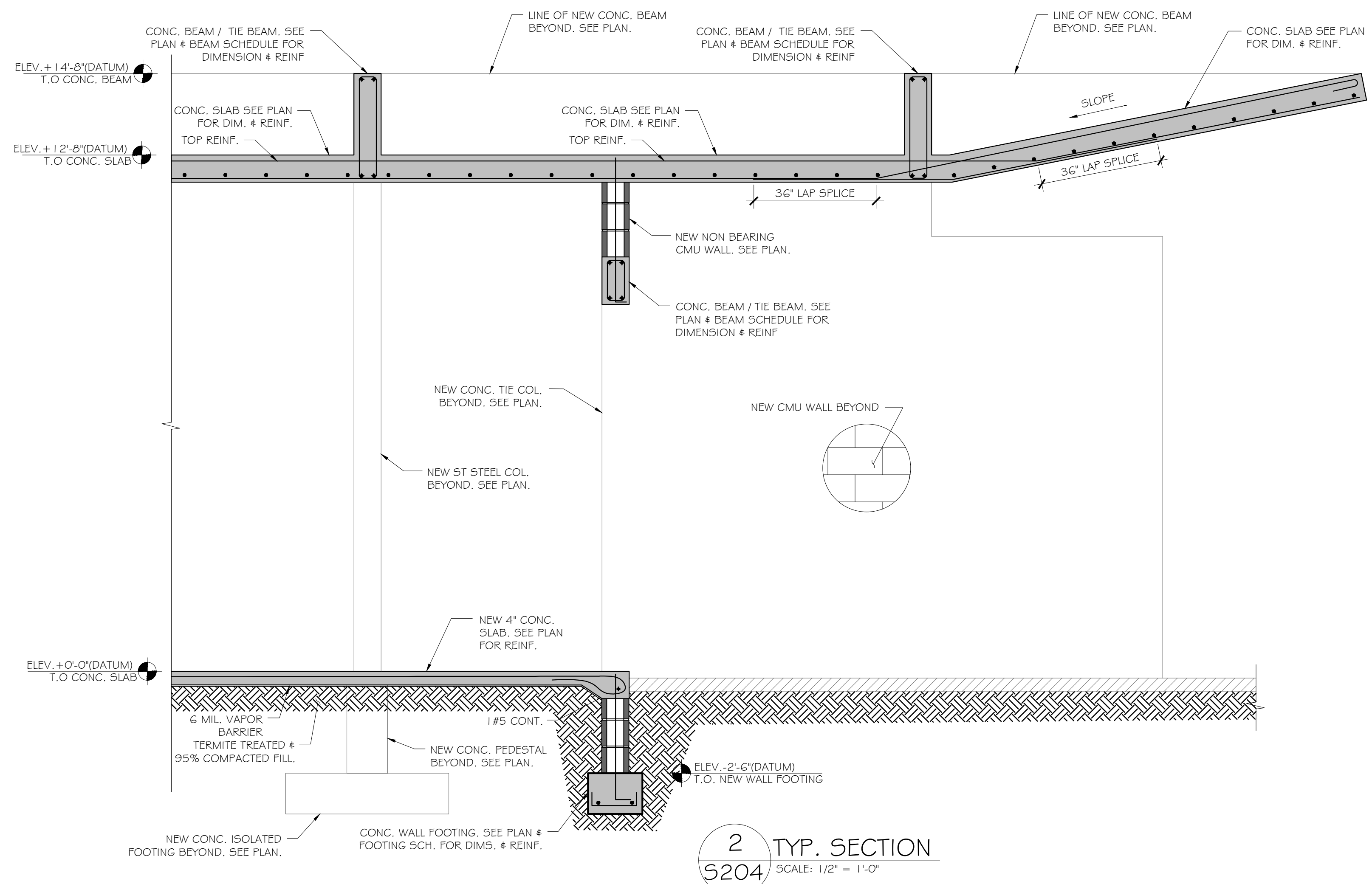


SIDE (R) FACADE
SCALE: 3/16" = 1'-0"

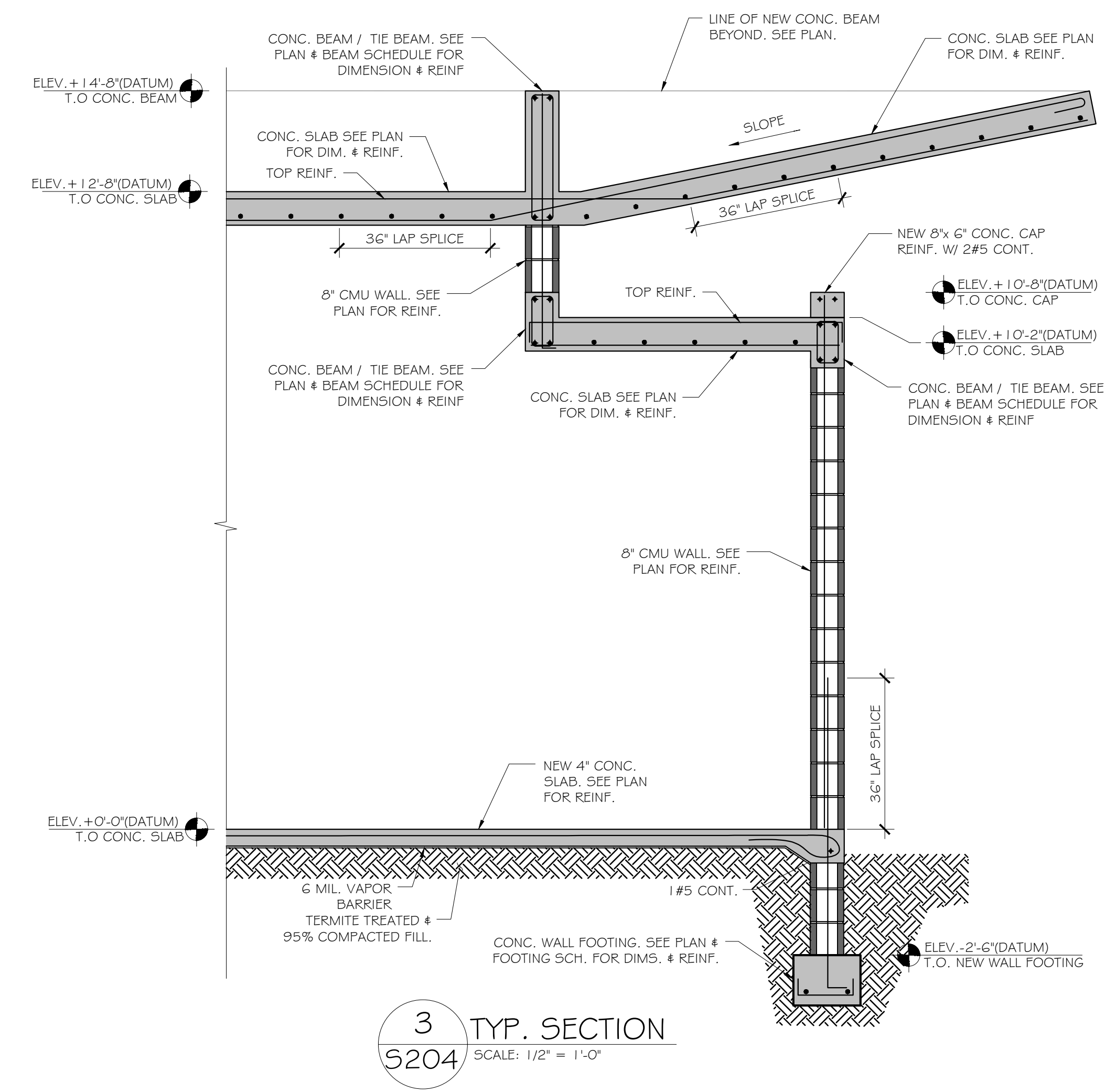
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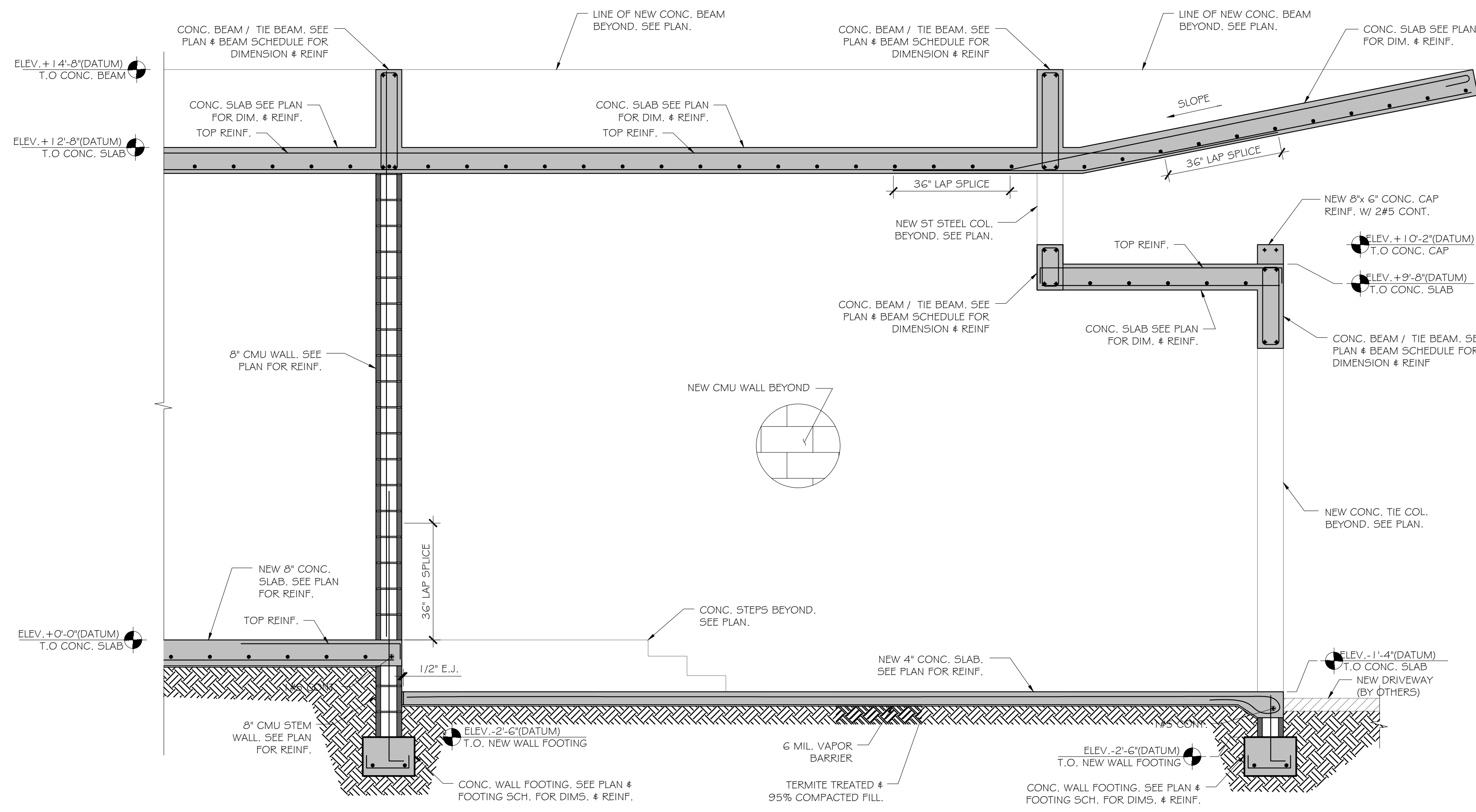
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2 TYP. SECTION
S204 SCALE: 1/2" = 1'-0"



3 TYP. SECTION
S204 SCALE: 1/2" = 1'-0"



1 TYP. SECTION
S204 SCALE: 1/2" = 1'-0"

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CONCRETE ROOF BEAM SCHEDULE (UPPER ROOF)

MARK	ELEV. (T.O.B)	SIZE B x D	BOTT. CONT.		TOP CONT.		"C" BARS		"E" BARS		STIRRUPS	REMARKS
			No.	SIZE	No.	SIZE	No.	SIZE	No.	SIZE		
RB-1	+14'-8"	8" x VAR.*	2	6	2	6	2	8	2	8	# 3 @ 15" O.C.	VARIES (W/8" # H-from 32" to 8") UP-TURNED BEAM
RB-2	+14'-8"	8" x 32"	2	6	2	6	2	7			# 3 @ 15" O.C.	UP-TURNED BEAM
RB-3	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-4	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-5	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-6	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-7	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-8	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-9	+14'-8"	8" x 32"	2	6	2	6	2	8	2	8	# 3 @ 15" O.C.	UP-TURNED BEAM
RB-10	+14'-8"	8" x 32"	2	6	2	6	2	8			# 3 @ 15" O.C.	UP-TURNED BEAM
RB-11	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-12	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-13	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM
RB-14	+14'-8"	8" x 32"	2	6	2	6					# 3 @ 15" O.C.	UP-TURNED BEAM

CONCRETE BEAM SCHEDULE (LOWER ROOF)

MARK	ELEV. (T.O.B)	SIZE B x D	BOTT. CONT.		TOP CONT.		"C" BARS		"I" BARS		STIRRUPS	REMARKS
			No.	SIZE	No.	SIZE	No.	SIZE	No.	SIZE		
2B-1	+10'-8"	8" x 20"	2	6	2	6					# 3 @ 7" O.C.	SEMI UP-TURNED BEAM
2B-2	+10'-8"	8" x 20"	2	6	2	6					# 3 @ 7" O.C.	SEMI UP-TURNED BEAM
2B-3	+10'-0"	8" x 12"	2	6	2	6					# 3 @ 7" O.C.	
2B-4	+10'-8"	8" x 16"	2	6	2	6					# 3 @ 7" O.C.	SEMI UP-TURNED BEAM
2B-5	+10'-2"	16" x 14"	2	6	2	6					# 3 @ 7" O.C.	
2B-6	+10'-8"	8" x 20"	2	6	2	6					# 3 @ 7" O.C.	UP-TURNED BEAM
2B-7	+10'-8"	8" x 14"	2	6	2	6					# 3 @ 9" O.C.	SEMI UP-TURNED BEAM
2B-8	+10'-8"	8" x 14"	2	6	2	6					# 3 @ 9" O.C.	UP-TURNED BEAM
2B-9	+10'-8"	8" x 14"	2	6	2	6					# 3 @ 9" O.C.	UP-TURNED BEAM
2B-10	+10'-8"	8" x 20"	2	6	2	6					# 3 @ 7" O.C.	UP-TURNED BEAM
2B-11	+10'-0"	8" x 12"	2	6	2	6					# 3 @ 7" O.C.	
2B-12	+10'-2"	8" x 14"	2	6	2	6					# 3 @ 7" O.C.	
2B-13	+11'-0"	8" x 18"	2	6	2	6					# 3 @ 7" O.C.	UP-TURNED BEAM
2B-14	+9'-8"	8" x 26"	2	6	2	6					# 3 @ 7" O.C.	
2B-15	+10'-2"	8" x 14"	2	6	2	6					# 3 @ 7" O.C.	UP-TURNED BEAM
2B-16	+10'-2"	8" x 14"	2	6	2	6					# 3 @ 7" O.C.	UP-TURNED BEAM
2B-17	+10'-8"	8" x 14"	2	6	2	6					# 3 @ 7" O.C.	UP-TURNED BEAM
2B-18	+10'-8"	8" x 14"	2	6	2	6					# 3 @ 7" O.C.	UP-TURNED BEAM
TB	SEE PLAN	8" x 12"	2	5	2	5					4#3@12"@CORNERS;BAL 40"O.C	PROVIDE #3@5" O.C AT OPENINGS

CONCRETE BEAM SCHEDULE (GROUND FLOOR)

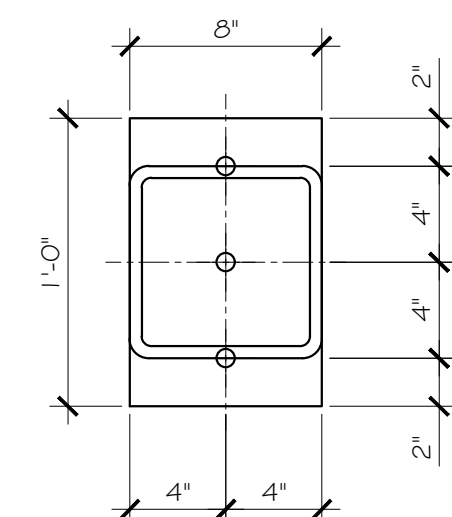
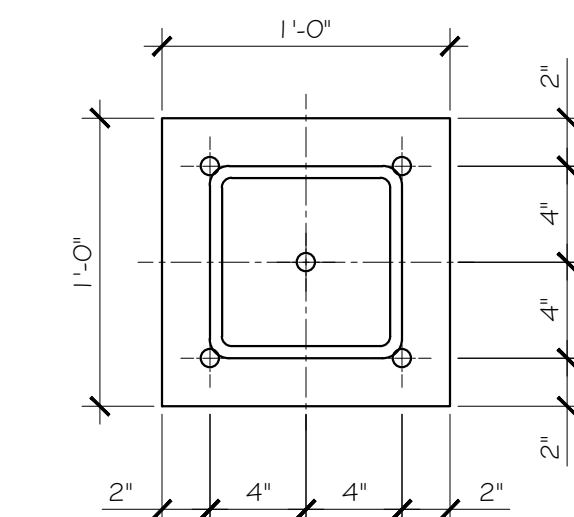
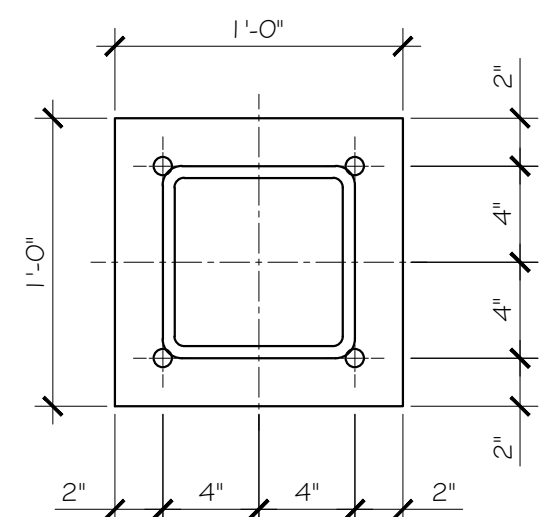
MARK	ELEV. (T.O.B)	SIZE B x D	BOTT. CONT.		TOP CONT.		"C" BARS		"I" BARS		STIRRUPS	REMARKS
			No.	SIZE	No.	SIZE	No.	SIZE	No.	SIZE		
1B-1	+2'-0"	8" x 24"	2	6	2	6					# 3 @ 7" O.C.	UP-TURNED BEAM

FOOTING SCHEDULE

FOOTING	DIMENSION	BOTTOM REINFORCEMENT	TOP REINFORCEMENT	REMARKS
WF-1G	16" x 12" x CONT.	3#5 CONT.		#3 @ 24" O.C. TRANSVERSAL
F-4	4'-0" x 4'-0" x 12"	5#5 E.W.	5#5 E.W.	
F-6	6'-0" x 6'-0" x 16"	7#5 E.W.		
F-3xG	3'-0" x 6'-0" x 16"	4#6 LONG & 7#5 SHORT	4#6 LONG & 7#5 SHORT	
F-3x8	3'-0" x 8'-0" x 16"	4#6 LONG & 7#5 SHORT	4#6 LONG & 7#5 SHORT	
TSE	12" x 10" x CONT.	1#5 CONT.		SEE TYP. DETAIL ON S-1.1

CONCRETE COLUMN SCHEDULE

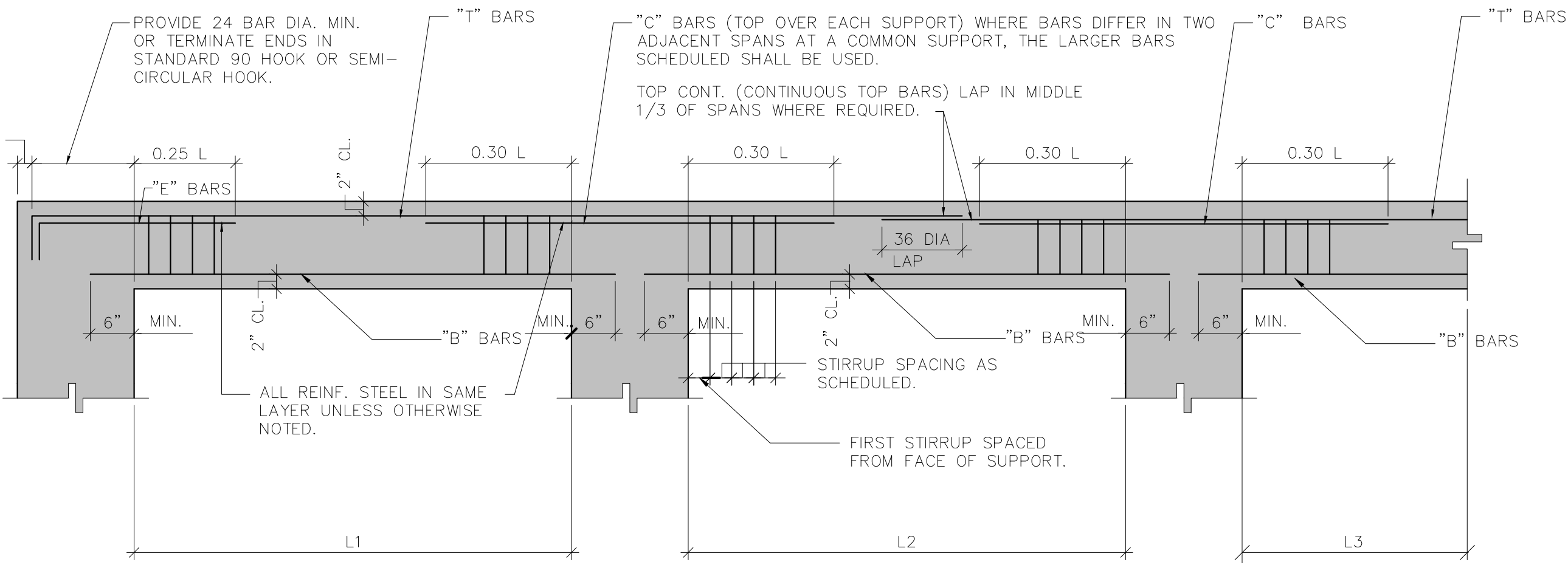
MARK	DIMS	LONG REINF.	TRANSV REINF.	REMARKS
TC	8" x 12"	4 # 5	#3 @ 8" O.C.	CONCRETE COLUMN
TC1	8" x 12"	4 # 5	#3 @ 8" O.C.	CONCRETE COLUMN
TC2	8" x 12"	4 # 5	#3 @ 8" O.C.	CONCRETE COLUMN
TC3	8" x 12"	4 # 5	#3 @ 8" O.C.	CONCRETE COLUMN
TC4	8" x 12"	4 # 5	#3 @ 8" O.C.	CONCRETE COLUMN



A TYP. PLATE DETAIL
SCALE: 1/2" = 1'-0"

B TYP. PLATE DETAIL
SCALE: 1/2" = 1'-0"

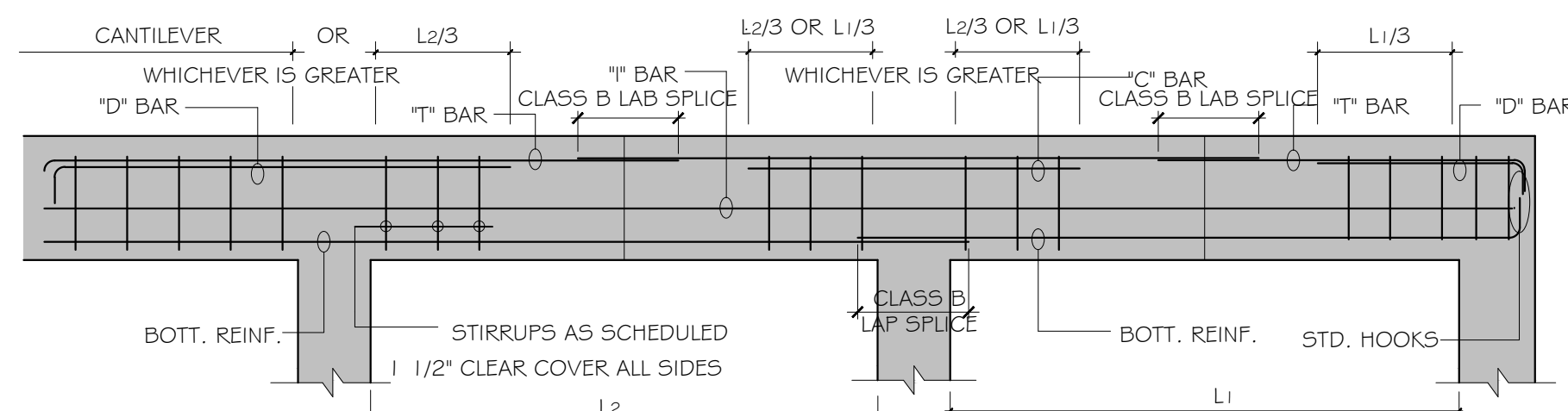
C TYP. PLATE DETAIL
SCALE: 1/2" = 1'-0"



NOTES:

- "L" INDICATES THE LARGER OF ADJACENT SPANS.
- WHERE BOTTOM BARS ARE INDICATED AS CONTINUOUS WITH ADJACENT SPANS, SPLICES, WHERE NEEDED, SHALL BE MADE IN THE END 1/3 OF SPAN WITH 24 DIA. LAP.

TYPICAL CONC. BEAM REINF. DIAGRAM FOR INT. BEAMS.

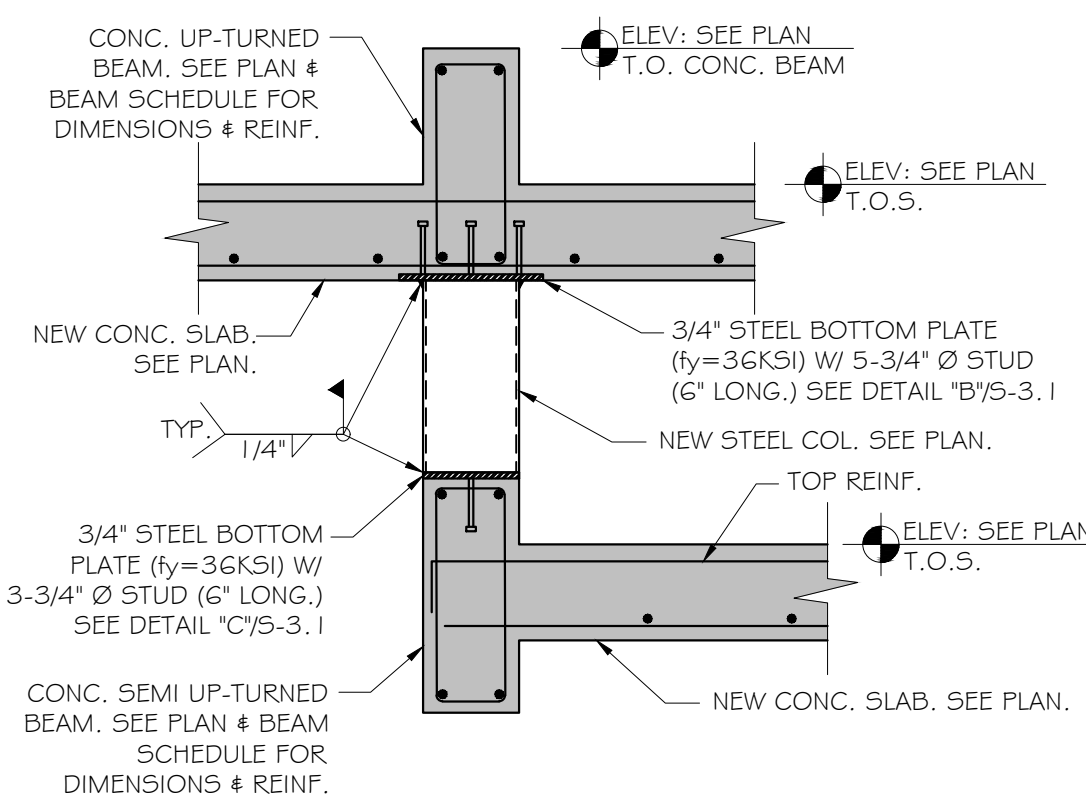
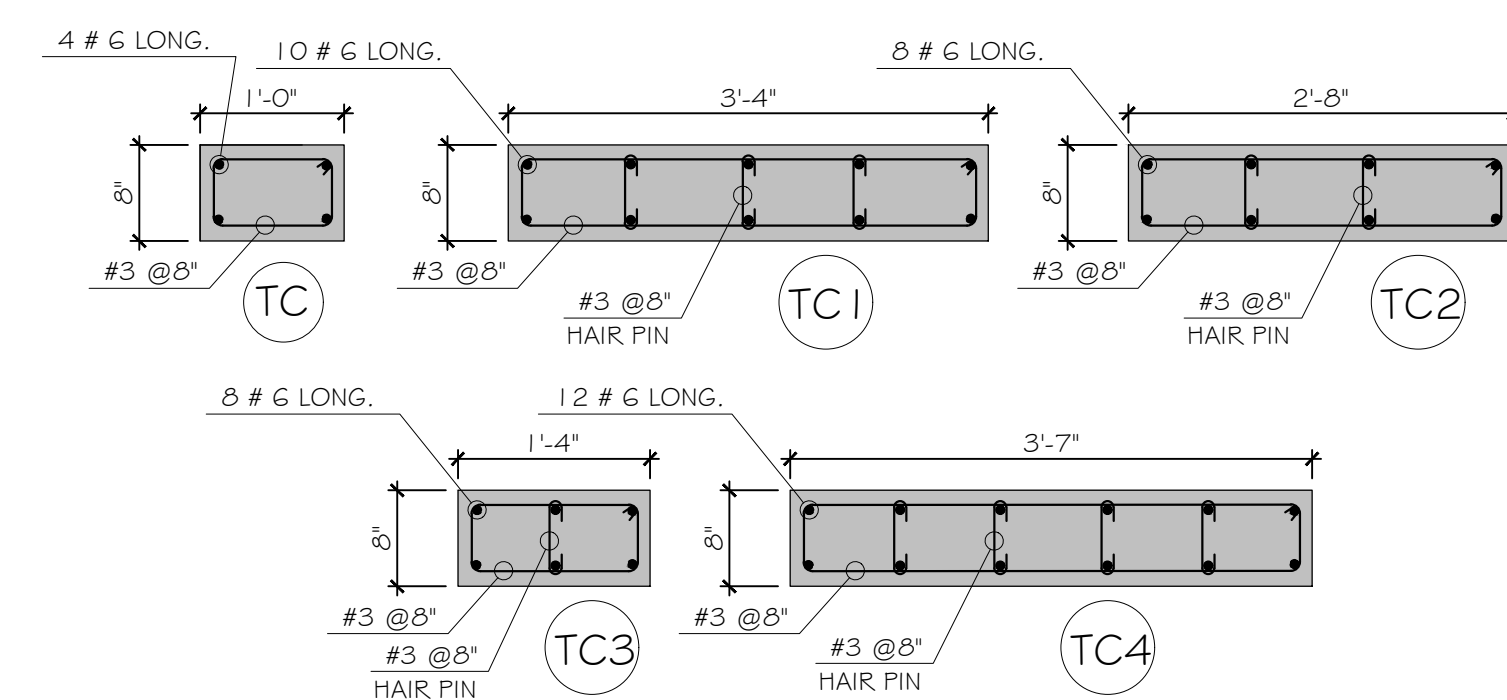


- "C" BARS ARE TOP BARS OVER RIGHT INTERIOR SUPPORTS SHALL BE EXTENDED TO THIRD SPAN ON BOTH SIDE
- "D" BARS ARE TOP BARS AT NON-CONTINUOUS ENDS OR AT CANTILEVER.
- "I" BAR ARE TOP BARS CALLED FOR AS CONTINUOUS, WHEN SPICED, LAP SPLICE SHALL BE CLASS B, AT LEAST ONE SIXTH OF TOTAL REINF. BUT NOT LESS THAN 2 BARS
- "I" INTERMEDIATE REINFORCEMENT. CLASS "B" SPLICE = 1.3ld

TYPICAL CONC. BEAM REINF. DIAGRAM FOR EXTERIOR BEAMS

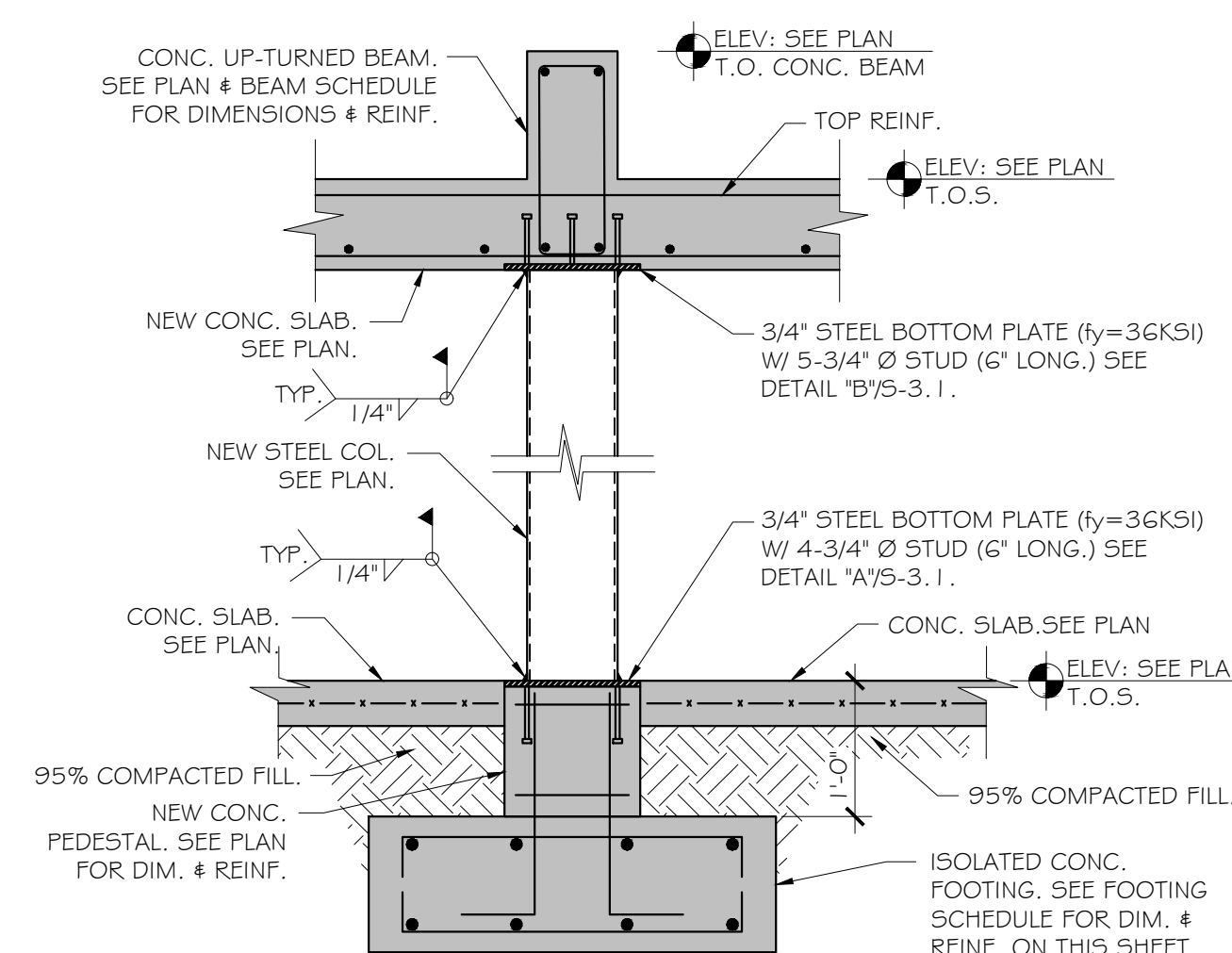
COLUMN DIAGRAM

SCALE: 3/4" = 1'-0"



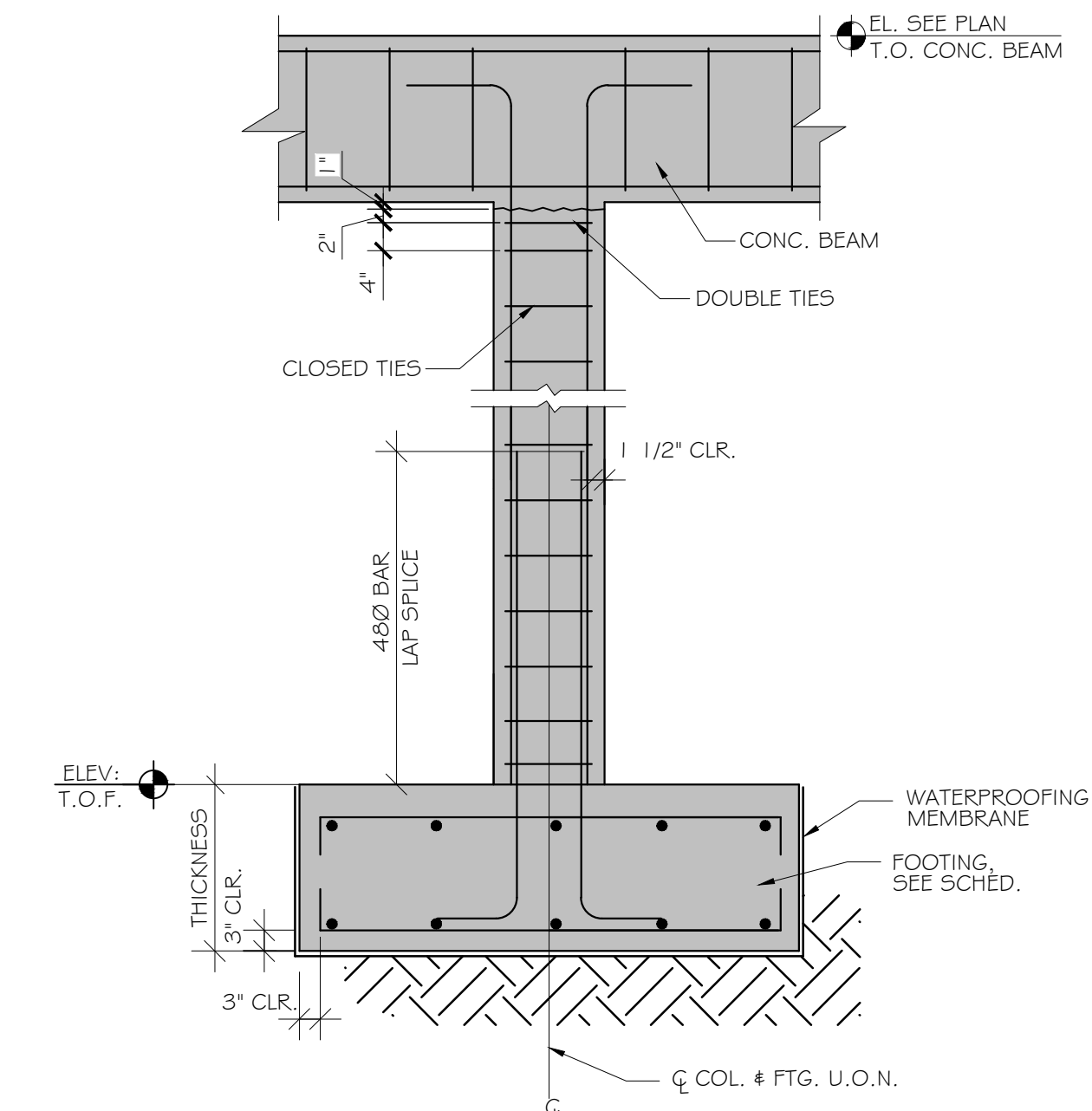
TYP. DETAIL OF STEEL COLUMN (SC1)

NOTE: EXTERIOR STEEL COLUMN AND PLATES MUST BE HOT DIPPED GALVANIZED. SCALE: 3/4" = 1'-0"



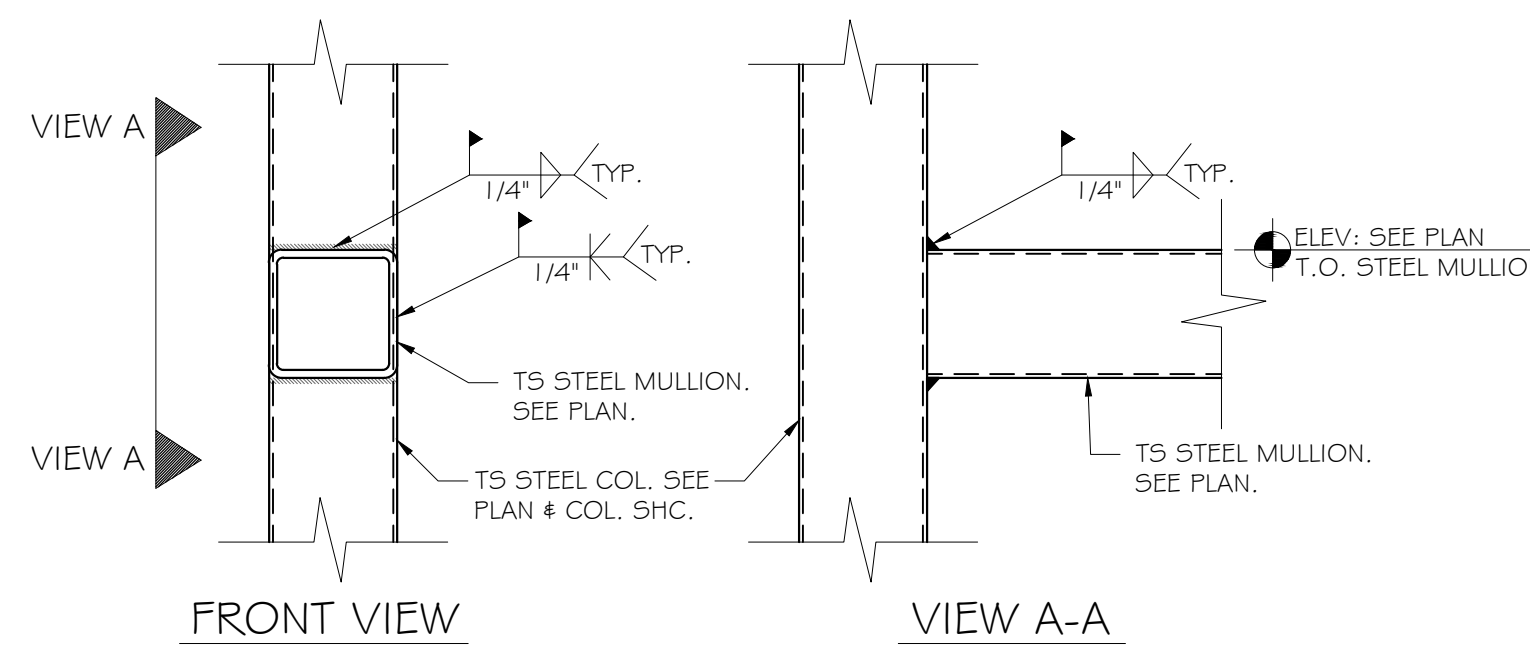
TYP. DETAIL OF STEEL COLUMN (SC)

NOTE: EXTERIOR STEEL COLUMN AND PLATES MUST BE HOT DIPPED GALVANIZED. SCALE: 3/4" = 1'-0"

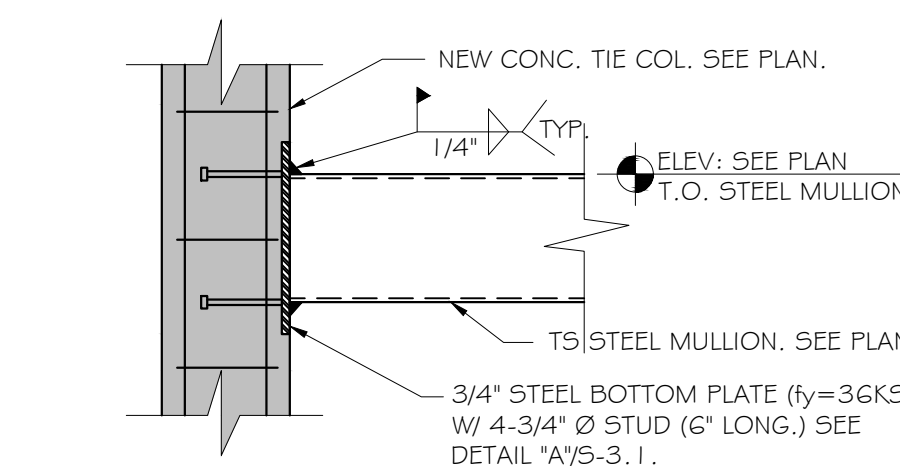


TYPICAL FOOTING-COLUMN-BEAM DETAIL

SCALE: 3/4" = 1'-0"



D TYP. TS STEEL COL. TO TS STEEL MULLION CONN. DETAIL
SCALE: 1" = 1'-0"



E TYP. CONC. COL. TO TS STEEL MULLION CONN. DETAIL
SCALE: 1" = 1'-0"

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STRUCTURAL NOTES

1. FOUNDATION:
 - A. FOUNDATION SYSTEM CONSISTS OF SPREAD FOOTINGS DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF.
 - B. SOIL STATEMENT: SOIL AT THIS SITE IS SAND & ROCK ADEQUATE TO SUPPORT THE DESIGN LOAD OF 2000 PSF. AFTER EXCAVATION SIGNED AND SEALED LETTER WILL BE SUBMITTED BY THE ARCHITECT OR THE ENGINEER ATTESTING THAT THE SITE HAS BEEN OBSERVED AND THE FOUNDATION CONDITIONS ARE SIMILAR TO THOSE WHICH THE DESIGN IS BASED ON.
 - C. ALL EDGE DISTANCES SHALL BE MAINTAINED IN ACCORDANCE WITH THE STRUCTURAL PLANS REGARDLESS OF THE MINIMUM TOLERANCES.
2. GENERAL:
 - A. ALL MATERIALS AND CONSTRUCTION SHALL COMPLY WITH THE FLORIDA BUILDING CODE, 2020 EDITION, ASCE 7-16 MINIMUM DESIGN LOADS FOR BUILDINGS, THE ACI 318-14 BUILDING CODE, AND ALL APPLICABLE FEDERAL, STATE AND LOCAL ORDINANCES.
 - B. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS OF EXISTING STRUCTURES AFFECTING NEW CONSTRUCTION BEFORE COMMENCING ANY WORK. ANY VARIATIONS IN ACTUAL FIELD CONDITIONS/DIMENSIONS FROM THOSE SHOWN IN THE CONTRACT DRAWINGS SHALL BE REPORTED TO THE ARCHITECT/ENGINEER FOR DETERMINING THE NEED OF REDESIGN PRIOR TO CONTRACTOR'S SUBMITTAL OF SHOP WORKING DRAWINGS FOR REVIEW.
 - C. THESE DRAWINGS SHALL BE WORKED TOGETHER WITH ARCHITECTURAL, AIR CONDITIONING, MECHANICAL AND ELECTRICAL DRAWINGS TO LOCATE DEPRESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, REGLETS, BOLT SETTINGS, SLEEVES, ETC. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK.
 - D. WHEN PERFORMING WORK BELOW GRADE, CARE SHALL BE TAKEN TO AVOID DAMAGING ANY EXISTING UTILITIES. ALL UNKNOWN UTILITIES DISCOVERED DURING CONSTRUCTION SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER. ANY DAMAGE TO THE EXISTING UTILITIES SHALL BE REPORTED TO ALL AFFECTED PARTIES, INCLUDING THE ARCHITECT/ENGINEER.
 - E. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR UPDATING HIS CONSTRUCTION DOCUMENTS WITH THE REVISED DRAWINGS AND SPECIFICATIONS, FIELD ORDERS, CHANGE ORDERS AND CLARIFICATION SKETCHES ISSUED DURING THE COURSE OF CONSTRUCTION.
 - F. TYPICAL DETAILS AND NOTES ON THESE DRAWINGS SHALL APPLY UNLESS SPECIFICALLY NOTED OTHERWISE. CONSTRUCTION DETAILS AND SECTIONS NOT COMPLETELY SHOWN OR NOTED SHALL BE SIMILAR TO DETAILS AND SECTIONS SHOWN OR NOTED FOR SIMILAR CONDITIONS.
 - G. THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.
 - H. BACKFILL AROUND THE EXTERIOR PERIMETER OF WALLS SHALL NOT BE PLACED UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. DO NOT PROCEED WITH BACKFILL UNTIL (7) DAYS AS A MINIMUM AFTER THE COMPLETION OF INTERIOR FLOOR SYSTEM UNLESS WALLS ARE ADEQUATELY BRACED. BACKFILL SHALL NOT BE PLACED UNTIL AFTER COMPLETION AND INSPECTION OF WATERPROOFING WHERE WATERPROOFING OCCUR.
 - I. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER FROM EXCAVATIONS AND DEWATERING OPERATIONS IN SUCH A WAY AS TO NOT CAUSE INCONVENIENCE TO THE WORK AND DAMAGE TO THE STRUCTURAL ELEMENTS.
 - J. REINFORCING STEEL SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT AND ARE SUPERSEDED BY THE STRUCTURAL DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST STRUCTURAL DRAWINGS.
 - K. THE CONTRACTOR SHALL SUPPLY THE ENGINEER THREE COPIES OF SHOP DRAWINGS A MINIMUM OF ONE WEEK PRIOR TO PLACEMENT. THE REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS ONLY FOR GENERAL COMPLIANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THE REVIEW DOES NOT GUARANTEE IN ANY WAY THAT THE SHOP DRAWINGS ARE CORRECT NOR DOES IT INFER THAT THEY SUPERSEDE THE STRUCTURAL DRAWINGS.
 - L. SUBMITTALS TO STRUCTURAL ENGINEER:
 - I. CONCRETE TEST REPORT FOR CAST-IN-PLACE CONCRETE AS PER 318-14.
 - II. REINFORCING STEEL SHOP DRAWINGS.
 - III. WOOD TRUSSES LAYOUT, SHOP DRAWINGS AND ALL DESIGN DATA.

5. REINFORCING STEEL:
 - A. REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH FBC 2020 EDITION.
 - B. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A 615-2010 (S1) GRADE 60.
 - C. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185-94.
 - D. REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE.
 - E. ALL TOP REINFORCING SHALL TERMINATE WITH STANDARD HOOKS AT DISCONTINUOUS EDGES OR ENDS.
 - F. ALL BOTTOM BARS SHALL BEAR 6" MINIMUM OVER SUPPORTS, U.O.N.
 - G. ALL REINFORCING BARS MARKED CONTINUOUS SHALL BE LAPPED 30 DIA. AT SPLICES AND CORNERS UNLESS OTHERWISE NOTED. LAP CONTINUOUS TOP BARS AT CENTER BETWEEN SUPPORTS AS REQUIRED. TERMINATE CONTINUOUS BARS AT NON-CONTINUOUS ENDS WITH STANDARD HOOKS, U.O.N.
 - H. MINIMUM CONCRETE COVER FOR REINFORCEMENT:
 - I. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.....3"
 - II. CONCRETE EXPOSED TO EARTH OR WEATHER #6 BARS AND LARGER.....2"
 - III. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH: SLABS AND WALLS.....3/4"
 - IV. EXTERIOR CONCRETE BALCONY SLABS.....2"

6. STRUCTURAL STEEL:
 - A. STRUCTURAL STEEL SHALL COMPLY WITH AISC 360 "SPECIFICATIONS FOR DESIGN, FABRICATION AND ERECTION FOR STRUCTURAL STEEL BUILDINGS, LAST EDITION 2020 (7TH EDITION).
 - B. STRUCTURAL STEEL SHAPES AND PLATES SHALL CONFORM TO ASTM A 36, Fy 36 KSI.
 - C. STRUCTURAL STEEL TUBES SHALL CONFORM TO ASTM A500, GRADE B, Fy=50 KSI.
 - D. STRUCTURAL STEEL PIPES SHALL CONFORM TO ASTM A53, TYPE S, GRADE B, Fy=35 KSI.
 - E. ANCHOR BOLTS SHALL CONFORM TO EITHER ASTM A 307 OR ASTM A 36.
 - F. FRAMING BOLTS SHALL CONFORM TO ASTM 325, WITH HARDENED WASHERS AND HEX NUTS.
7. WELDING:
 - A. WELDING SHALL BE DONE WITH E-70 ELECTRODES.
 - B. WELDERS SHALL BE AWS-CERTIFIED.
8. WOOD FRAMING:
 - A. WOOD FRAMING MEMBERS OTHER THAN TRUSSES SHALL BE #2 SOUTHERN PINE WITH A FIBER BENDING STRESS AS PER NDS.
 - B. SIZES SHOWN ARE NOMINAL.
 - C. MEMBERS SHALL BE FREE OF CRACKS AND KNOTS.
 - D. MOISTURE CONTENT SHALL BE 19% OR LESS.
 - E. PRESSURE-TREATED WOOD SHALL BE USED ONLY WHERE SPECIALLY NOTED IN THE DRAWINGS. NO FRAMING MEMBER SHALL BE OF PRESSURE-TREATED WOOD, UNLESS OTHERWISE NOTED.
9. BALCONY AND TERRACE RAILINGS AND STAIR RAILINGS SHALL BE DESIGNED BY MANUFACTURER'S REGISTERED ENGINEER IN THE STATE OF FLORIDA TO RESIST A LOAD OF 50LBS/FT. APPLIED IN ANY DIRECTION AT TOP OF SUCH BARRIER. POSTS SHALL BE DESIGNED TO RESIST THE REACTION FROM THE RAILINGS OR A MINIMUM LOAD OF 200 LBS. HANDRAILS SHALL BE DESIGNED AND CONSTRUCTED TO RESIST A LOAD OF NOT LESS THAN 200 LBS. APPLIED IN ANY DIRECTION AND AT ANY POINT ON THE RAIL.
10. ROOF SHEATHING:
 - A. PLYWOOD ROOF SHEATHING SHALL HAVE A NET THICKNESS OF NOT LESS THAN 5/8" WHEN THE SPAN IS NOT MORE THAN 24" AND SHALL HAVE STAGGERED JOINTS CONTINUOUS OVER TWO OR MORE SPANS WITH FACE GRAIN PERPENDICULAR TO SUPPORTS.
 - B. PLYWOOD ROOF SHEATHING SHALL BE NAILED WITH 1 1/4" RING SHANK NAILS @ 2 1/2" O.C. WITH SPACING NOT EXCEED 2 1/2" O.C. AT PANEL EDGES AND AT INTERMEDIATE SUPPORTS/BLOCKING AND 4". ELSE WHERE IF APPLICABLE. PROVIDE 2X4 IN. BLOCKING AT RIDGE AND HIP LINES W/ 2 - 1 1/4" TOE NAILS FROM EACH SIDE, NAIL PLYWOOD TO BLOCKING AT 4 IN. OC.

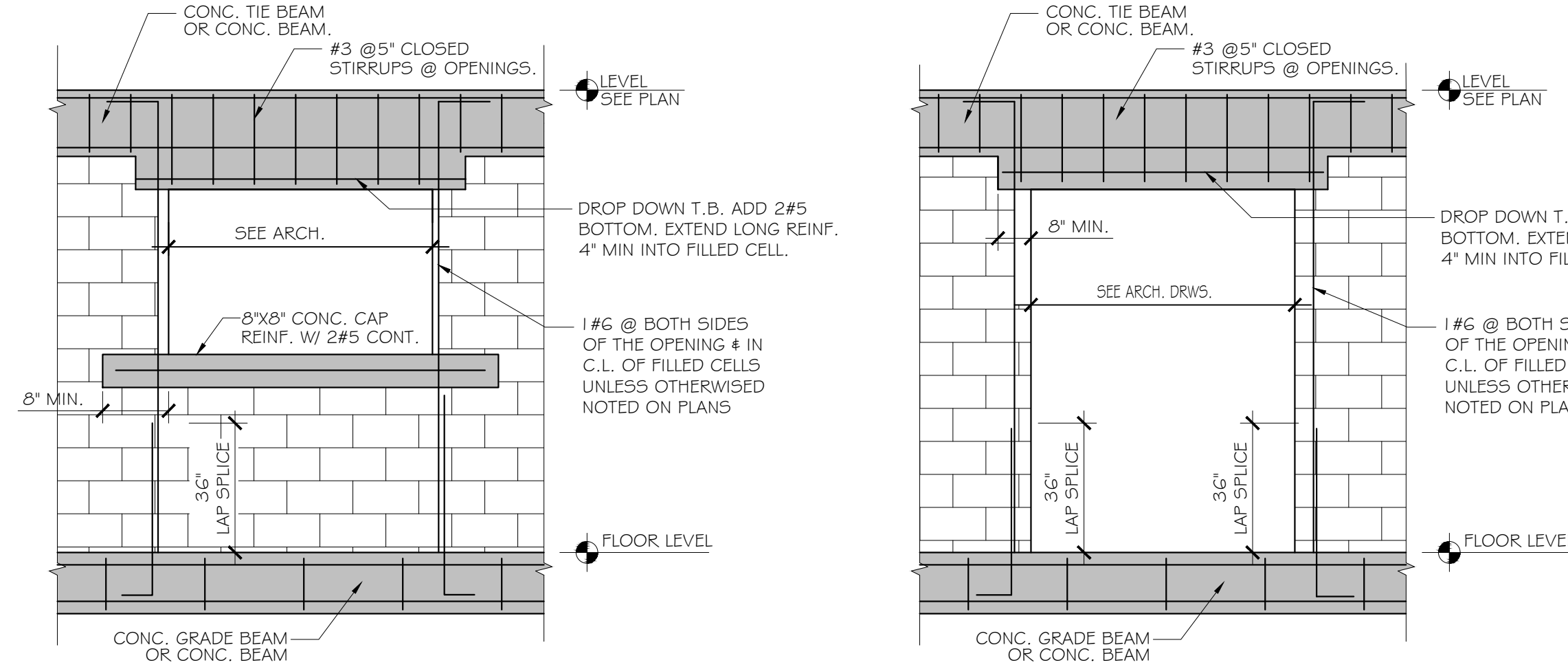
3. CONCRETE:
 - A. ALL CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301-2010 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."
 - B. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 5000 PSI FOR GRADE BEAMS, COLUMNS, SLABS AND ROOF BEAMS.
 - C. FORMWORK SHALL COMPLY WITH ACI 347-04. "RECOMMENDED PRACTICE FOR CONCRETE WORK."
 - D. NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE.
 - E. THE OWNER SHALL CONTRACT AN INDEPENDENT TESTING LABORATORY TO PERFORM CONCRETE CYLINDER TESTS AS FOLLOWS: FOUR CYLINDER TESTS FOR ANY 50 CUBIC YARDS OF CONCRETE POURED, OR THREE CYLINDER TESTS PER ANY DAYS POUR LESS THAN 50 CUBIC YARDS. ONE CYLINDER SHALL BE TESTED AT 7 DAYS AND TWO AT 28 DAYS.
 - F. TRANSPORTING, PLACING, CURING AND DEPOSITING OF CONCRETE SHALL COMPLY WITH ACI 301-96.
 - G. CONCRETE MIX FOR BALCONIES SHALL HAVE ALL CORROSION INHIBITING ADDITIVES & A MINIMUM CONCRETE COMPRESSIVE STRENGTH OF 3000 PSI
4. CONCRETE MASONRY WORKS:
 - A. ALL CONCRETE MASONRY WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 530-11.
 - B. ALL CONCRETE MASONRY WALLS ARE DESIGNED AS LOAD BEARING WALLS, UNLESS OTHERWISE NOTED, AND SHALL BE IN PLACE BEFORE THE SLABS AND BEAMS SUPPORTED BY THEM ARE POURED AS WELL AS THE CONCRETE IS FRAMING THEM.
 - C. ALL CONCRETE MASONRY UNITS (C.M.U.) SHALL CONFORM TO ASTM C 90 "STANDARD SPECIFICATIONS FOR HOLLOW LOA BEARING CONCRETE MASONRY UNITS".
 - D. MORTAR SHALL CONFORM TO ASTM C 270, TYPE "M", WITH A MINIMUM AVERAGE STRENGTH OF 2500 PSI.
 - E. CONCRETE MASONRY STRENGTH, f_m , SHALL BE A MINIMUM OF 1500 PSI ($f_c = 1900$ PSI).
 - F. VERTICAL REINFORCING IN C.M.U. CELLS SHALL BE SPLICED WITH 48 BAR DIAMETER LAP SPLICE, PROVIDE CLEAN OUT HOLES AT BASE OF FILLED CELLS FOR LAP INSPECTION AND VERIFYING THAT THE CELLS HAVE BEEN FILLED SOLID WITH GROUT.
 - G. FILLED CELLS SHALL BE FILLED WITH 3000 PSI GROUT AS PER ACI 530-11 AND ACI 530-1-11. FILLING OF CELLS SHALL BE DONE IN FOUR FOOT LIFTS WITH A MAXIMUM POUR OF 12 FEET. USE MECHANICAL VIBRATION TO ACHIEVE GROUT-FILLED SOLID CELLS. GROUT SHALL CONFORM TO ASTM C-476. SLUMP SHALL BE BETWEEN 8" AND 11".
 - H. ALL C.M.U. WALLS SHALL BE HORIZONTALLY REINFORCED WITH STANDARD NO. 9 LADDER-TYPE GALVANIZED STEEL REINFORCING EVERY SECOND COURSE. EXTEND REINFORCING A MINIMUM OF 4 INCHES INTO THE COLUMNS.

NOTES:

- I. All exterior ceilings must be constructed with 5/8" plywood. See sheathing notes for nailing.
- II. DESIGN CRITERIA: ROOF TRUSSES: DL: 25 PSF LL: 30 PSF

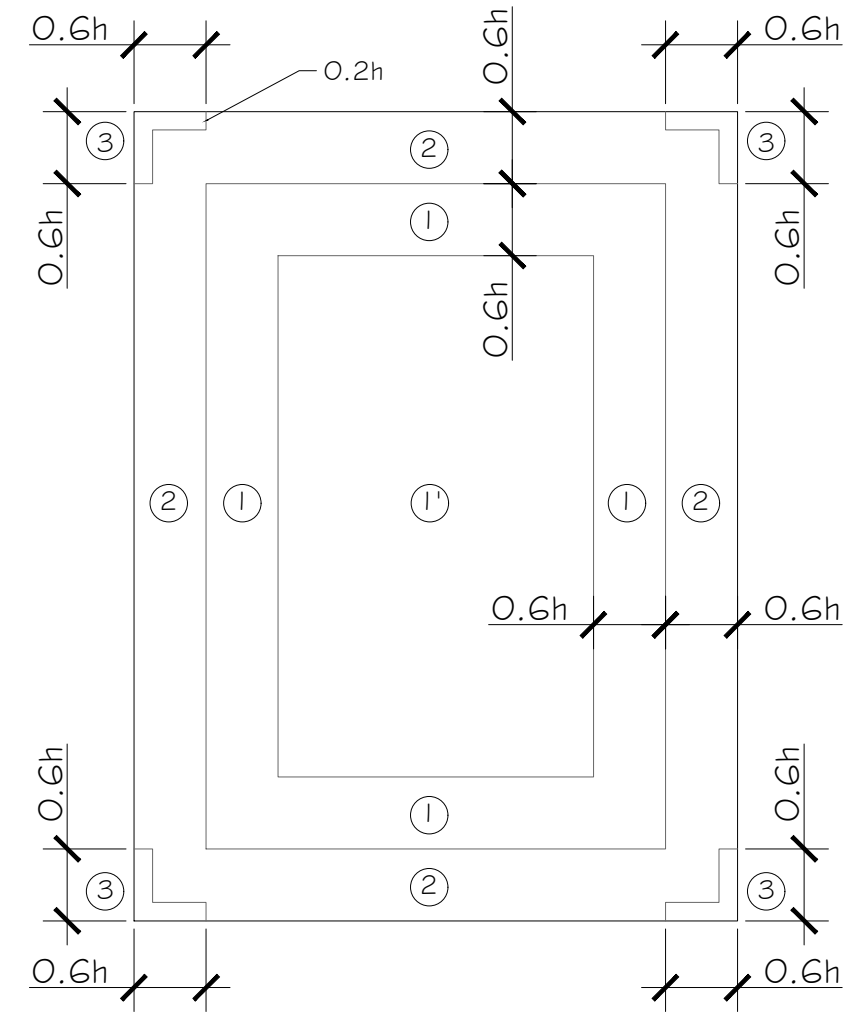
WIND LOAD: ASCE 7-16 WIND VELOCITY: 175 MPH EXPOSURE "C" CATEGORY "II"

IN MY PROFESSIONAL JUDGEMENT AND TO THE BEST OF MY KNOWLEDGE AND BELIEF, THESE PLANS AND SPECIFICATIONS COMPLY WITH THE FLORIDA BUILDING CODE, 7TH EDITION (2020).



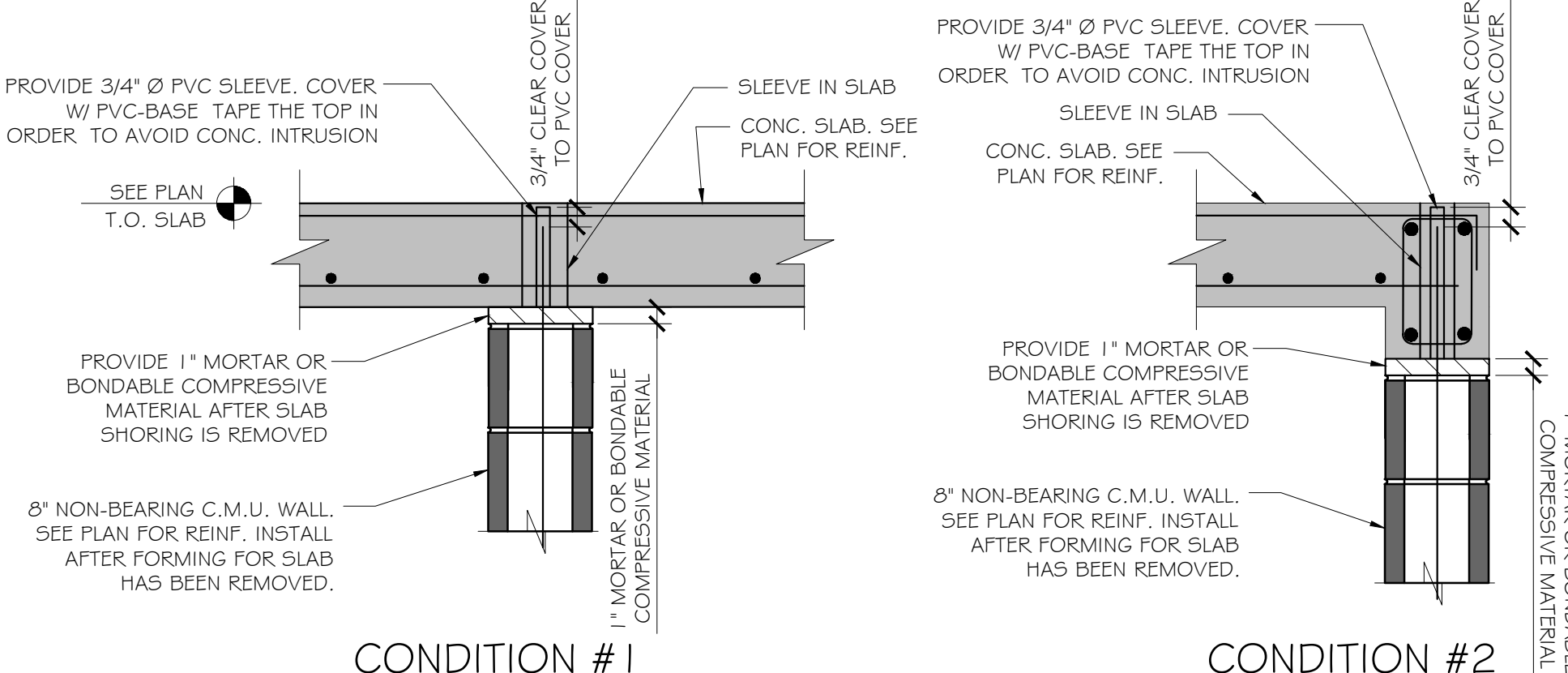
TYP. NEW FILL-CELL REINF. DETAIL @ WINDOW OPENINGS. SCALE: 3/4" = 1'-0"

TYP. NEW FILL-CELL REINF. DETAIL @ DOOR OPENINGS. SCALE: 3/4" = 1'-0"

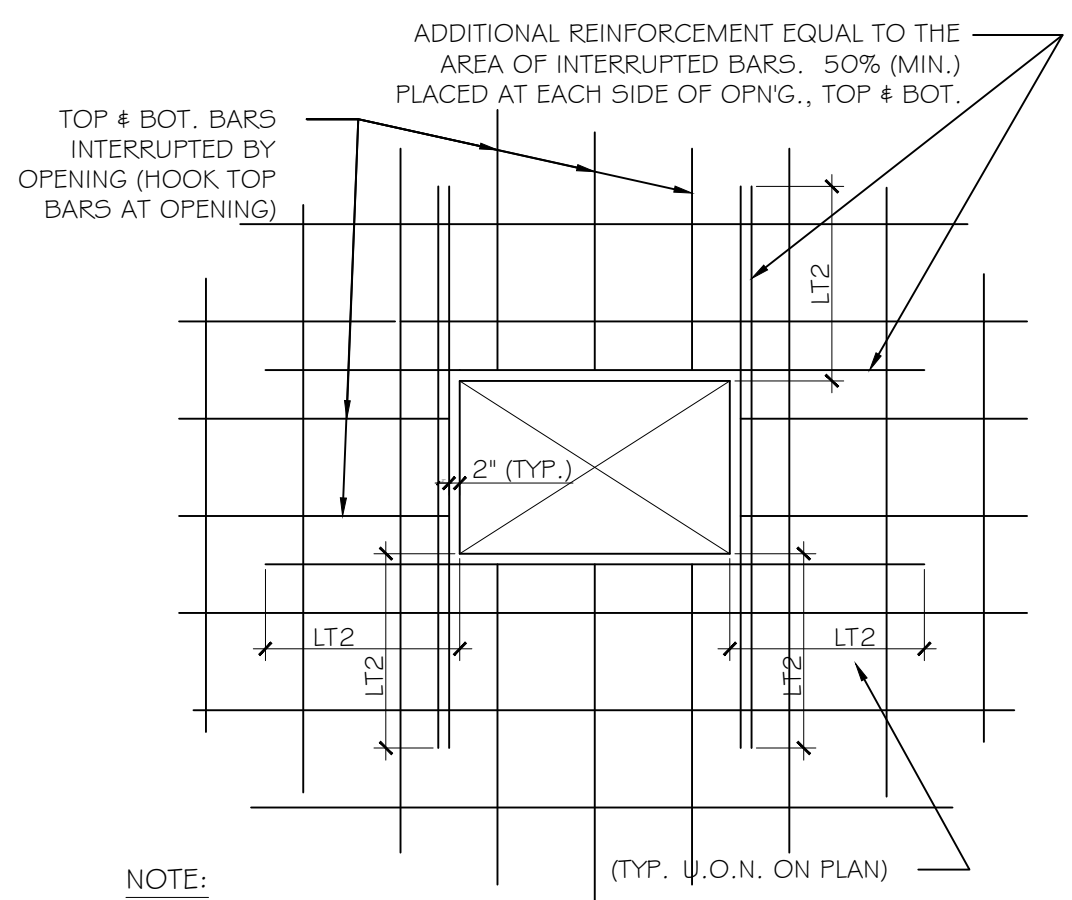


Roof Plan

ROOF WIND PRESSURES ON ROOFING SYSTEM (0.6W)		
ZONE 1	+19.77 PSF	-77.44 PSF
ZONE 1'	+19.77 PSF	-44.49 PSF
ZONE 2	+19.77 PSF	-102.16 PSF
ZONE 3	+19.77 PSF	-139.23 PSF
END ZONE LENGTH h = 14.0'		



CONDITION #1
CONDITION #2
A TYPICAL NON-BEARING WALL CONN. TO CONC. SLAB DETAIL
SCALE: 1" = 1'-0"



NOTE: IF SIZE OF OPENING IS LESS THAN TWICE THE SLAB THICKNESS, REINF. MAY BE ROUTED AROUND OPNG., UNLESS OTHERWISE SHOWN ON PLANS. IF SIZE OF OPENING IS MORE THAN TWICE THE SLAB THICKNESS, REINF. WILL BE INTERRUPTED AND REPLACED BY ADDITIONAL REINF. AS SHOWN IN THIS DETAIL, UNLESS OTHERWISE SHOWN ON PLANS.
TYP. U.O.N. ON PLAN
SCALE: 1/8" = 1'-0"

STRIP LOCATION	MINIMUM PERCENTAGE AT SECTION	WITHOUT DROP PANELS		WITH DROP PANELS	
		TOP	Bottom	TOP	Bottom
COLUMN STRIP	50	0.30 l_n	0.30 l_n	0.33 l_n	0.33 l_n
	Remainder	0.20 l_n	0.20 l_n	0.20 l_n	0.20 l_n
MIDDLE STRIP	100	0.22 l_n	0.22 l_n	0.22 l_n	0.22 l_n
	Remainder	6"	6"	6"	6"

Additional notes from diagram: Class A splices shall be permitted in this region. At least two bars or wires shall conform to 13.3.8.5. Max. 0.15 l . Clear span - l_n . Face of support. Center to center span - l . Exterior support (No slab continuity). Interior support (Continuity provided). Exterior support I (No slab continuity).

TYP. SLAB MILD REBAR REINF. DETAIL
NO SCALE

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NEED-24-HOURS